1) The inflow hydrograph of the spillway design flood for a proposed reservoir and the reservoir characteristics are given in the two tables below. The spillway will have a crest elevation of 56.4 m and a crest length, L of 24 m . Reservoir outflow, $\mathrm{Q}=2.0 \mathrm{Lh}^{1.5}$ where Q is in $\mathrm{m}^{3} / \mathrm{s}, \mathrm{L}$ is in m , and h is the depth of water above the spillway crest in m . Find the peak outflow and the maximum water surface elevation.

| Time $(\mathrm{min})$ | 0 | 30 | 60 | 90 | 120 | 150 | 180 | 210 | 240 | 270 | 300 | 330 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Inflow $\left(\mathrm{m}^{3} / \mathrm{s}\right)$ | 0 | 7.6 | 32 | 101.3 | 162.2 | 147.4 | 119.4 | 76.4 | 37.9 | 16.7 | 4.2 | 0 |


| Elevation (m) | 56.4 | 57.9 | 59.4 |
| :--- | :--- | :--- | :--- |

$\begin{array}{llll}\text { Surface area (ha) } & 28.5 & 32.7 & 37.2\end{array}$
2) A settling column test is conducted in a 2.40 m deep column. Sampling ports are located at $60 \mathrm{~cm}, 120$ cm and 180 cm . Samples were collected at the intervals and depths as indicated in the table below. If the initial solids concentration, $\mathrm{C}_{0}=430 \mathrm{mg} / \mathrm{L}$ and the surface overflow rate, $\mathrm{V}_{0}=0.0475 \mathrm{~m} / \mathrm{min}$, what percentage of solids will be removed? If you want to remove $60 \%$ of the solids, what should be the surface overflow rate? Although the column is 2.40 m deep, consider only a settling depth of 1.80 m to allow for sludge collection in the bottom 60 cm .

| Time (minutes) | 60 cm depth | 120 cm depth | 180 cm depth |
| :---: | :---: | :---: | :---: |
| 5 | 357 | 387 | 396 |
| 10 | 310 | 346 | 366 |
| 20 | 252 | 299 | 316 |
| 30 | 198 | 254 | 288 |
| 40 | 163 | 230 | 252 |
| 50 | 144 | 196 | 232 |
| 60 | 116 | 179 | 204 |
| 75 | 108 | 143 | 181 |

3) A flocculation basin required to handle $200,000 \mathrm{~m}^{3} / \mathrm{d}$ is 30 m long by 30 m wide by 4.5 m deep and it has 6 baffled compartments. Each compartment has a horizontal shaft supporting 6 paddle flocculators with four arms each, for a total of 36 units. A maximum of 5 blades, each 4.6 m by 0.15 m can be attached to each arm at radii of $2.00 \mathrm{~m}, 1.70 \mathrm{~m}, 1.40 \mathrm{~m}, 1.10 \mathrm{~m}$ and 0.80 m , from the center of the blades to the centerline of the shaft. The maximum rotation can provide a velocity of $0.80 \mathrm{~m} / \mathrm{s}$ at the radius of 2.0 m (to center of outer blade). For this paddle flocculator with paddles at multiple radii the equation below can be used where $\mathrm{k}=$ the ratio of water velocity to paddle velocity and in this case $\mathrm{k}=0.3$. The area of paddles A in the equation is the total area for paddles at only one radius. The drag coefficient is 1.8. If the water temperature is $10^{\circ} \mathrm{C}$ and the velocity gradient is $60 \mathrm{~s}^{-1}$, what is the minimum number of blades that should be installed? Assume that the same blade arrangement should be used for each of the 6 units.

$$
P=\frac{C_{d} A \rho(1-k)^{3}(2 \pi n)^{3}\left(r_{1}^{3}+r_{2}^{3}+r_{3}^{3}+\ldots\right)}{2}
$$

4) Design sedimentation basins with 5 m wide chain and flight sludge removal equipment if the maximum daily flow $\mathrm{Q}_{\max }=3.25 \mathrm{~m}^{3} / \mathrm{s}$, the surface loading rate $\mathrm{V}_{0}=2.0 \mathrm{~m} / \mathrm{h}$ and the water temperature T is $5^{\circ} \mathrm{C}$. Using a 90 degree V notch weir, determine the total horizontal weir length required to achieve an acceptable weir overflow rate WOR. Let your design conform to the table in the notes taken from "Water Principles, Treatment and Design". Also check your detention time for the average daily flow assuming that $\mathrm{Q}_{\mathrm{avg}} \times 1.5=\mathrm{Q}_{\text {max }}$.
