

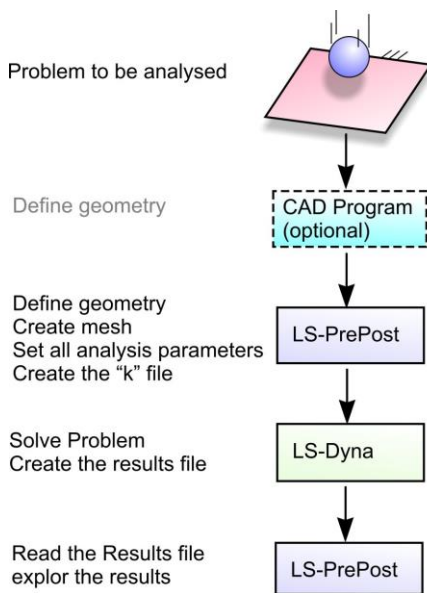
Tutorial #1

Introduction to LSDyna: Simple Cantilever

By C. Daley

Overview

LSDyna™ is a special type of finite element program produced by Livermore Software Technology Corporation (LSTC). The program is particularly well suited to problems involving impact and complex non-linear behaviour (eg buckling, plasticity, fracture). For this introduction, we will examine a simple cantilever bar. We will be demonstrating with the release 4.0 (Beta) of LS-PrePost. LS-PrePost is a free program that is used to prepare simulations and examine results. You can install this program on any computer you want (ie at home or on your laptop) and prepare models. The calculation program is called LSDyna. It is not free, but you can use it at the university. The sketch below shows the sequence of steps to perform a simulation:



Unlike the quasi-static simulations that are typically done in FE programs such as ANSYS, LSDYNA (normally) performs dynamic simulations, where time is actual time (in seconds or milliseconds). So even though we will start with a problem that could be solved quasi-statically, LSDyna will solve it dynamically. By dynamically, we mean that LSDyna will take the inertia of the elements and their accelerations into account. So all elements must have a specified density and mass, something not required for a quasi-static analysis in ANSYS. (Note - ANSYS-Explicit is like LSDyna)

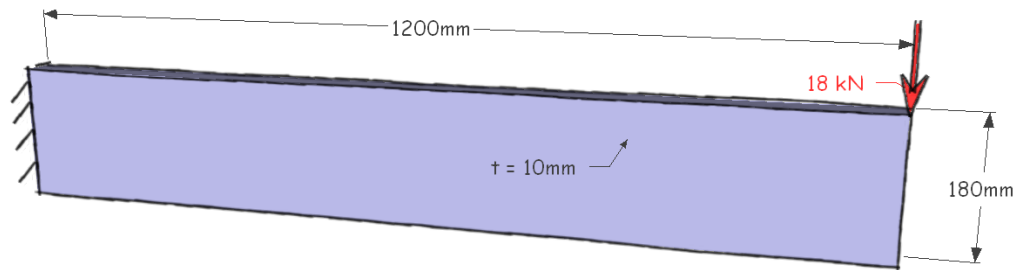
Learning Objectives

The lab is intended to show the basic user interface of LS-PrePost and LSDyna. The lab will cover the necessary steps to permit the simplest of structural analyses.

LSDYNA Model #1 – simple cantilever

Step 1: describe and sketch the problem:

In this first example we will model a simple steel cantilever, to see how the simple structure responds to load. The problem is sketched below.



The problem description is as follows:

Geometry: 1.200 x .180 x .010 m

Load: 18000 N applied at the end of the cantilever in the string direction

Supports: the base is fixed in all degrees of freedom, all other boundaries are free.

Material: Steel, with $E = 200e9$ Pa ($2e11$ N/m²), $\sigma_y = 3e8$ Pa, $E_t = 1e9$ Pa

Units: N, m, Pa, s NOTE: The user must use consistent units

Step 2: estimate expected results (analytically):

The bar has the following properties:

Moment of inertia : $I = 1/12 t h^3 = 10 \times 180^3 / 12 = 4.860e06$ mm⁴

Section Modulus: $Z = I/(h/2) = 54000$ mm³



Base Bending Moment: $M = 18000 \times 1200$ N-mm

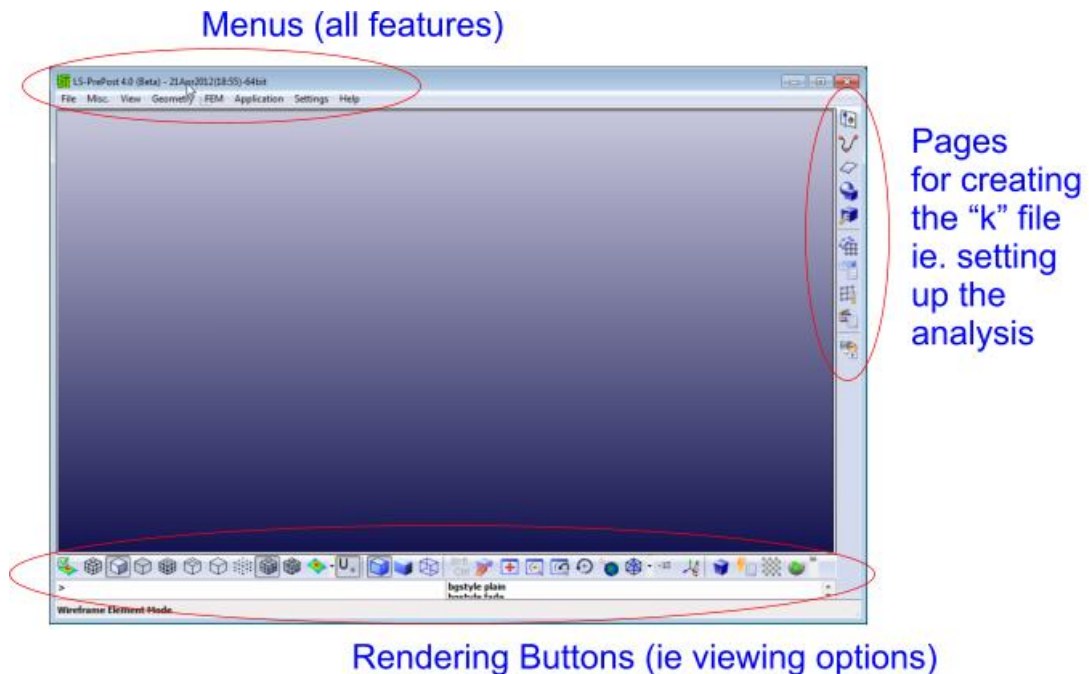
Maximum stress (at base): $\sigma = M/Z = 400$ N/mm² or MPa

Maximum deflection: $d = FL^3/(3 EI) = 10$ mm



It is likely that the LSDYNA results will be close to these, and close to the ANSYS analysis of this problem that we did in the course 5003. The % error will depend on the assumptions, but differences of say +/- 10% would not be unusual.

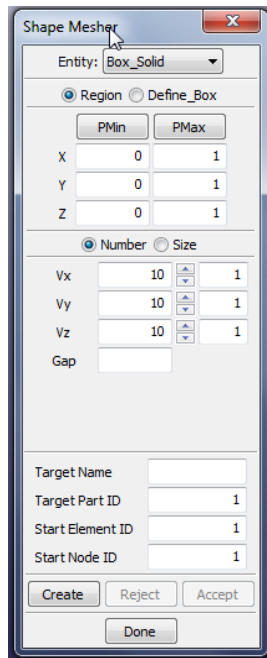
Step 3: open LS-PrePost

- 1) First, Open LS-PrePost which should be on the desktop, with Icon  or 
- 2) The window will be as shown below. There are pull down menus at the top of the screen, a variety of rendering buttons for display options at the bottom, and a set of modeling and editing icons on the right side. There is an older version of the interface that you can see by pressing F11. You might want that interface if you want to follow an old tutorial example (many are available on line). We will use the new GUI icon interface.



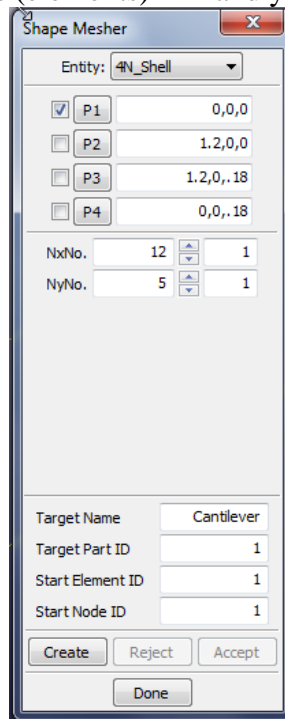
Step 4: Directly create the FE model (nodes and element mesh)

- 1) Click the **element and mesh** button: . which opens up a new set of icons for meshing.
- 2) Click the **shape mesher** button: . which opens a dialog window to directly create a mesh. (Note: in this case there will be no CAD model - we go straight to a finite element mesh)



The initial dialog look like:

In the **Entity** box at the top, select: **4N_Shell** and fill in the 4 corner coordinates, the number of divisions (elements) in x and y, and the **TargetName**,

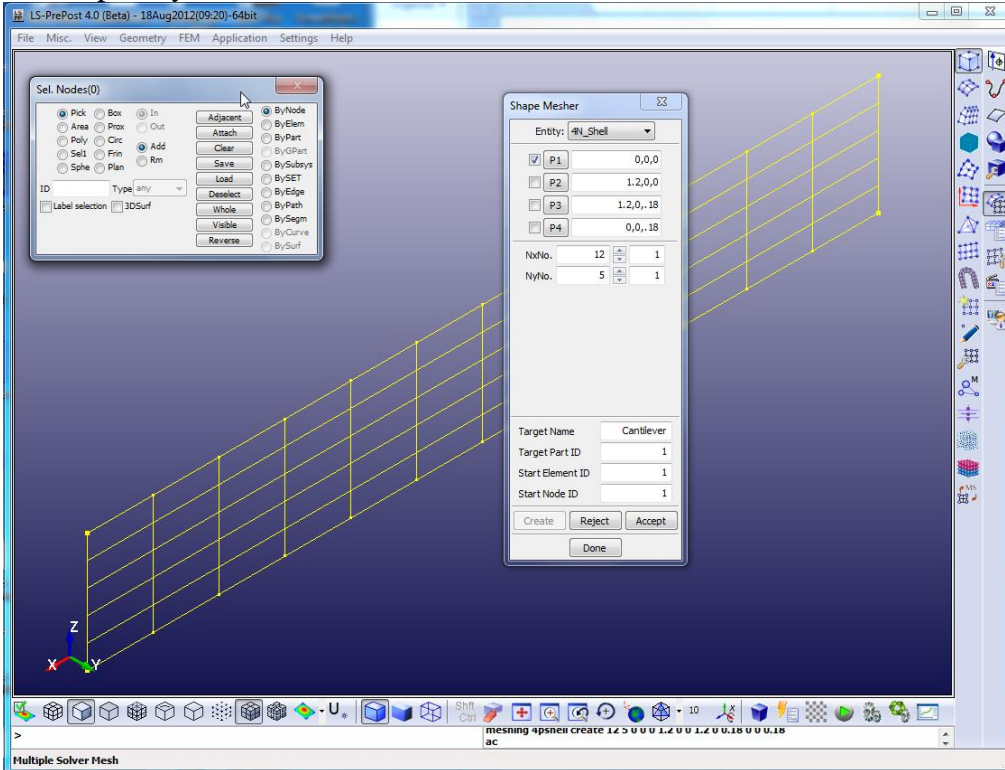


so it looks like this:

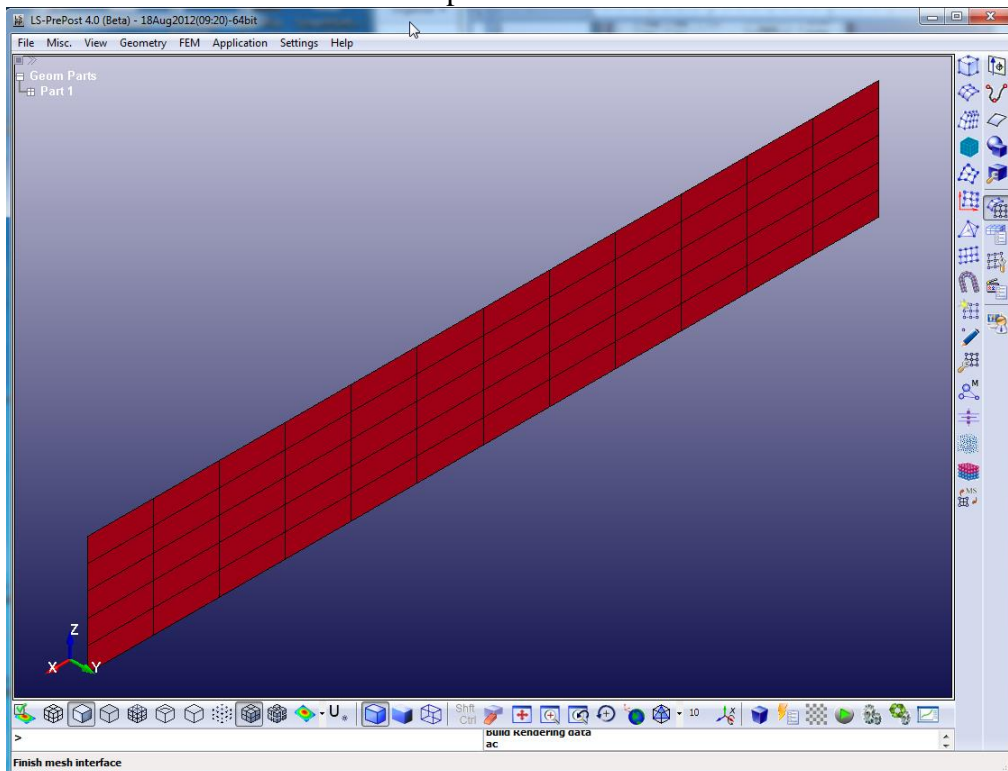
When you click on **Create**, a trial mesh is created. To see it you will need to change the view to isometric by clicking on the **IsoMetric** icon at the bottom of the LSDyna

window: 



At this point you should have a window that looks like:

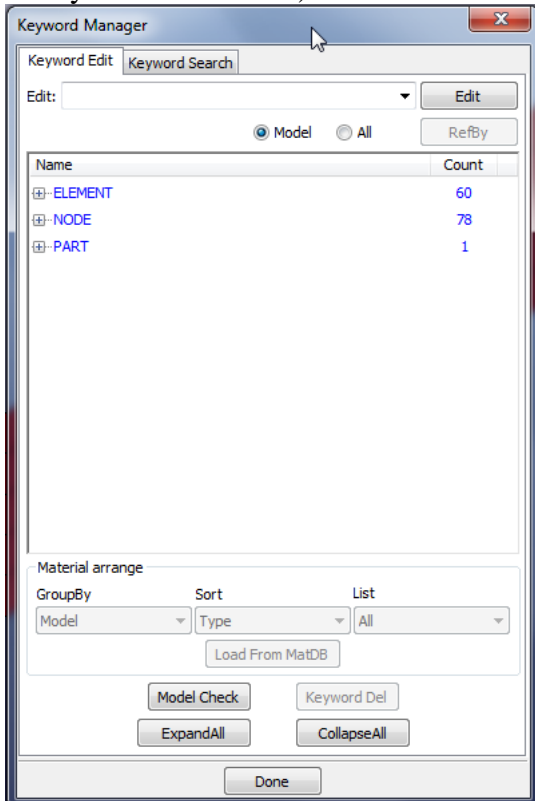


You can edit the values, and even reject the trial mesh. If it's ok you click **Accept** and **Done** to finalize the mesh. At this point the screen should look like:

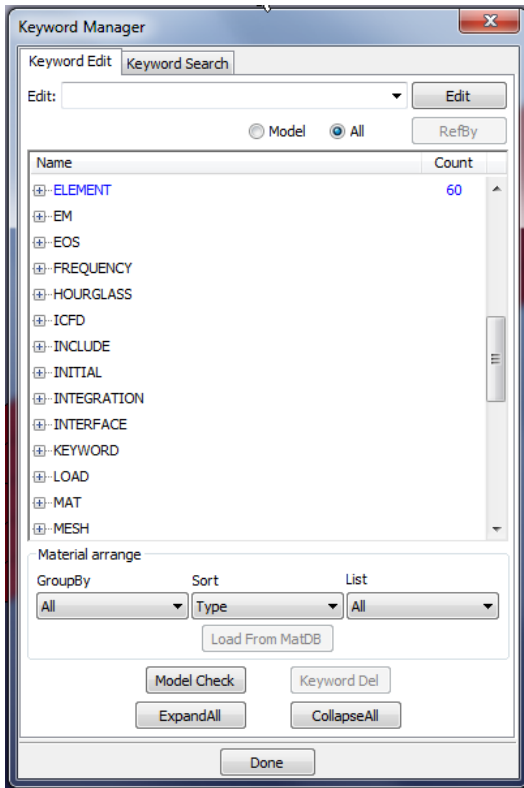


Step 5: Define the material properties

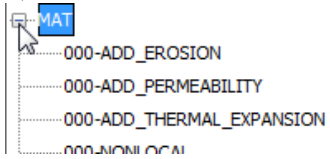
- 1) Click the **model and part** button:  which will bring up a new set of icons.
- 2) Click the **keyword manager** button:  which opens a dialog window to directly create keywords (Note: LS-Dyna files are called .k files because they are essentially a list of keyword commands). A window will open that looks like:



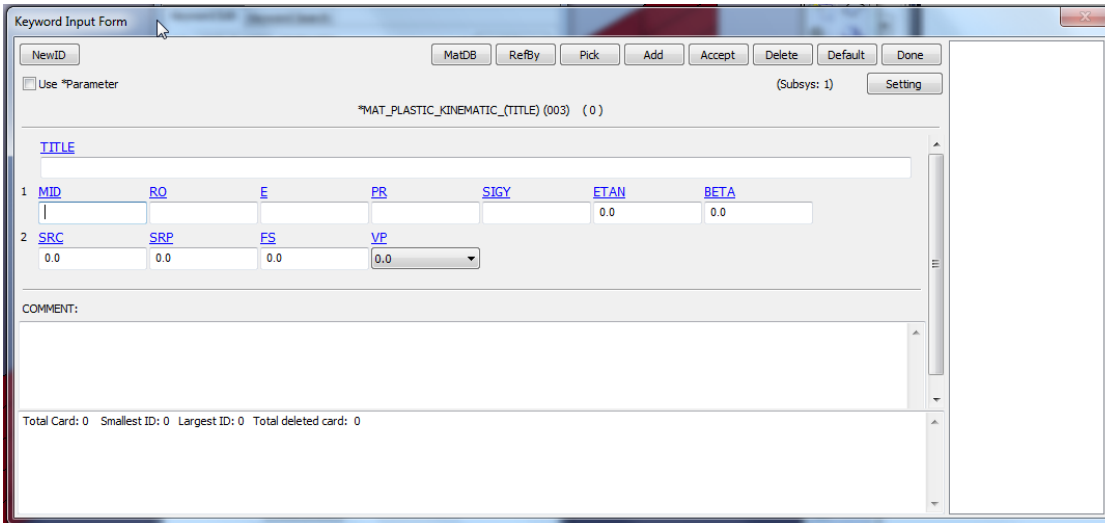
At this point the model option Model All is selected, and there are 3 categories of keyword already there (element, node and part). Note that there are 60 elements, along with the 78 nodes and 1 part. The part was created automatically when we created the mesh (a mesh has nodes and elements). Select the All option and the window will display all the possible keywords:



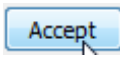
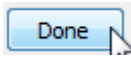
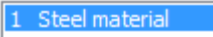
3) Click on the + to the left of **MAT** to open the material options:



4) Select **003-PLASTIC_KINEMATIC** and then click on **Edit** near the top of the window. A **Keyword Input Form** will open to allow input of the properties for this material model:

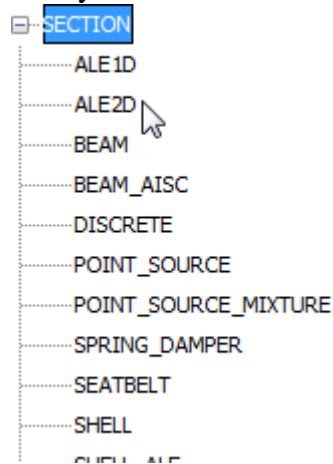


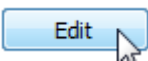
- 5) click on **NewID** and fill in the form. Enter values for **TITLE**, **RO**, **E**, **PR**, **SIGY**, **ETAN**. Other options are left at default values;

then press  and then press . After pressing Accept, you will see  in the right hand space of the form. This material model describes elasto-plastic behavior in steel. This is similar to the bilinear-kinematic material property that can be specified in ANSYS.

Step 6: Define the section properties

- 1) In LSDyna the section properties contain information about the element parameters and the type of physics employed. In the **Keyword Manager** window, scroll down until you see the **Section** Keyword. Click on the + and then select **Shell**



- 2) Click on  near the top of the window. A **Keyword Input Form** will open to allow input of the properties for this section model. click on **NewID** and fill in the form as follows:

The form shows that the plate elements will have a thickness of 0.010 m (10mm), that there are 5 integration points through the thickness (NIP=5) and the shear factor SHRF is 0.833. When you enter the first thickness T1, all other values (T2 etc) are made identical. These are the thicknesses of all for corners of this shell section. When complete, press **Accept** and then press **Done**. After pressing Accept, you will see **2 Shell Section** in the right hand space of the form.

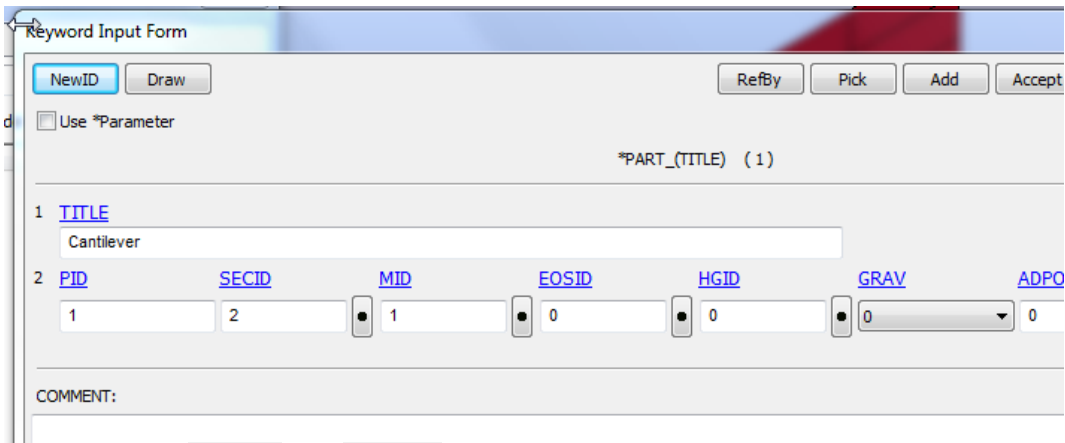
Step 7: Apply the material and section properties to the model (to the 'Part')



1) In the **Keyword Manager** window, select the **Part** Keyword, press + and select **Part**:

Then press **Edit**. The part is defined, but the section and material information is not linked to the part (there are zeros under SECID and MID);

Press the black dot to the right of SECID, and you see the following window open;

Select **2 Shell Section** and press **Done**. When you do the SECID becomes 2. Click on the black dot to the right of MID and select **1 Steel material** and press **Done**. With this the window will look like:



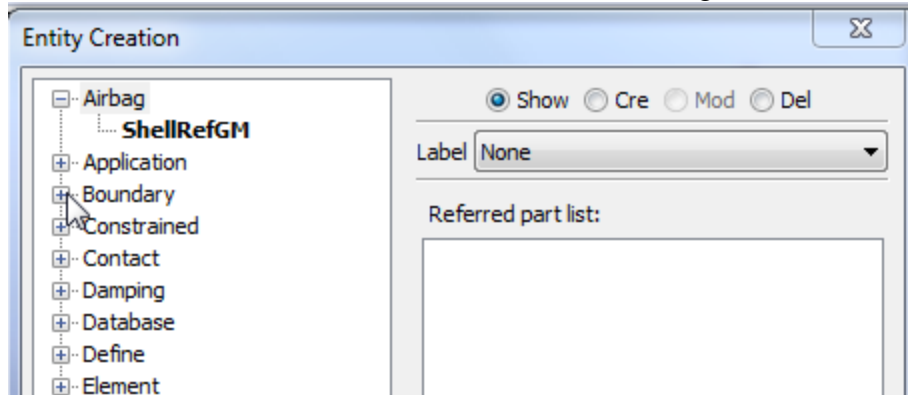
then press  and . We now have a finite element model with material and section properties.



Step 8: Apply boundary conditions (fix the end of the bar')


Rather than using the **Keyword Manager** window, we will use a special tool to create the boundary conditions (an app) :

The **Model and Part** button already selected: 

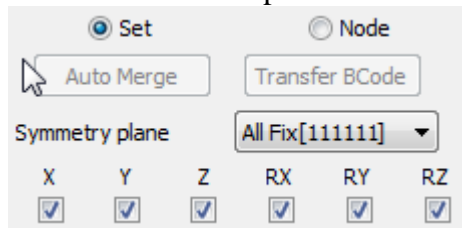
Click on the **Create Entity** button.  This will bring us an **Entity Creation** form.



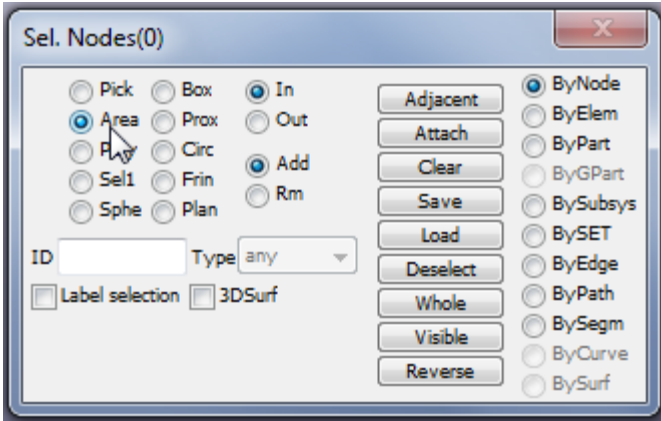
Select the + **Boundary** and then select **Spc** :  

Now change to Create mode by selecting 

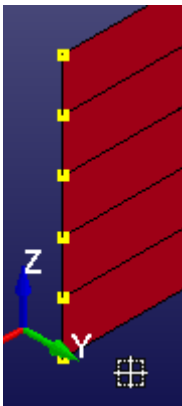
Make sure the **Set** option selected and all 6 degrees of freedom selected:



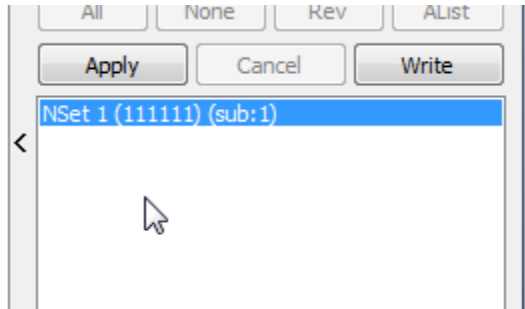
Now you need to select the nodes at the left end of the model. A **Set Nodes** dialog box should be open: Make sure the **Area** option is selected:



Now the cursor has changed to a selection box. Select all the nodes at the end of the model and hit :



You will see that a set of nodes has been created:





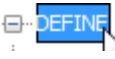

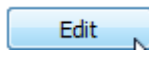
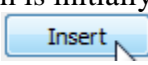
Now hit  to close this activity.

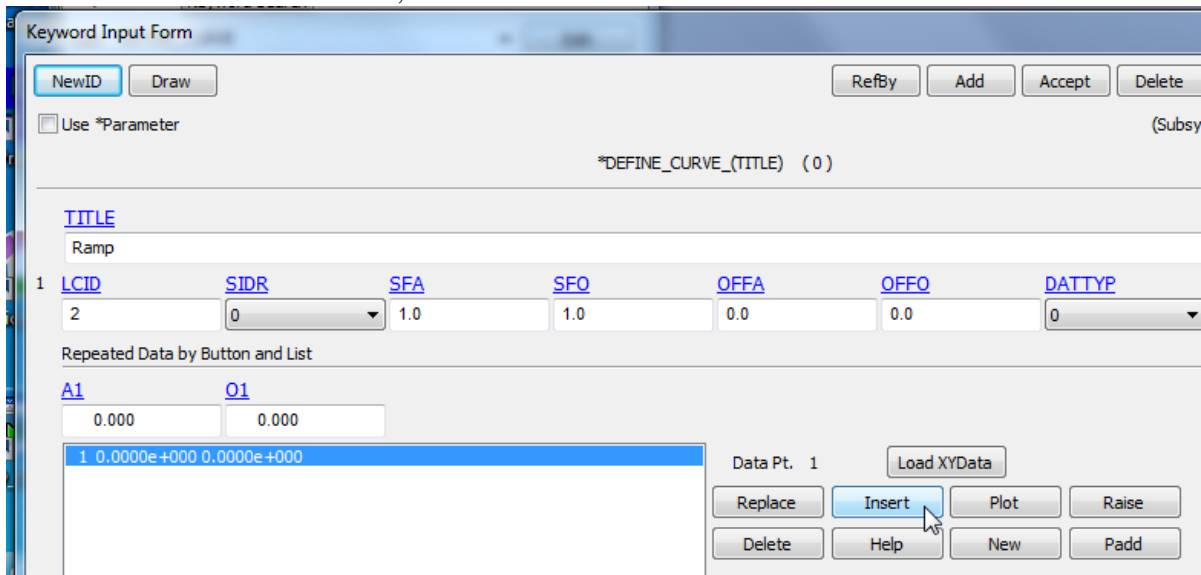
Step 9: Apply a force to the free end of the bar.

This activity requires a few sub-steps. We need to define a curve to tell LSDyna how to apply the force in time (remember - everything in LSDyna happens in real time). And we need to know the node number of the top right corner of the model.

1) Define a standard ramp curve:

Click the **model and part** button:  and click the **keyword manager** button: 

Now Scroll to **DEFINE**, open it (hit +)  and select **CURVE**  and then the **Edit**  key at the top. This will bring up a **Keyword Input Form** to create the curve. The form is initially blank. Hit **NewID**, give it the **Title Ramp** and with **A1** and **O1** both = 0.0 hit the **Insert**  button, The form will now look like this.

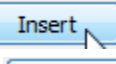


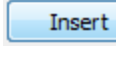
1	LCID	SDR	SFA	SFO	OFFA	OFFO	DATYP
	2	0	1.0	1.0	0.0	0.0	0

Repeated Data by Button and List

A1	O1
0.000	0.000

1	0.00000e+000	0.00000e+000
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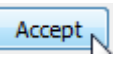
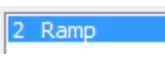
Now edit the **A1** and **O1** boxes to 1 and 1, and hit **Insert**  again.

Now edit the **A1** and **O1** boxes to 1.1 and 1, and hit **Insert**  again.

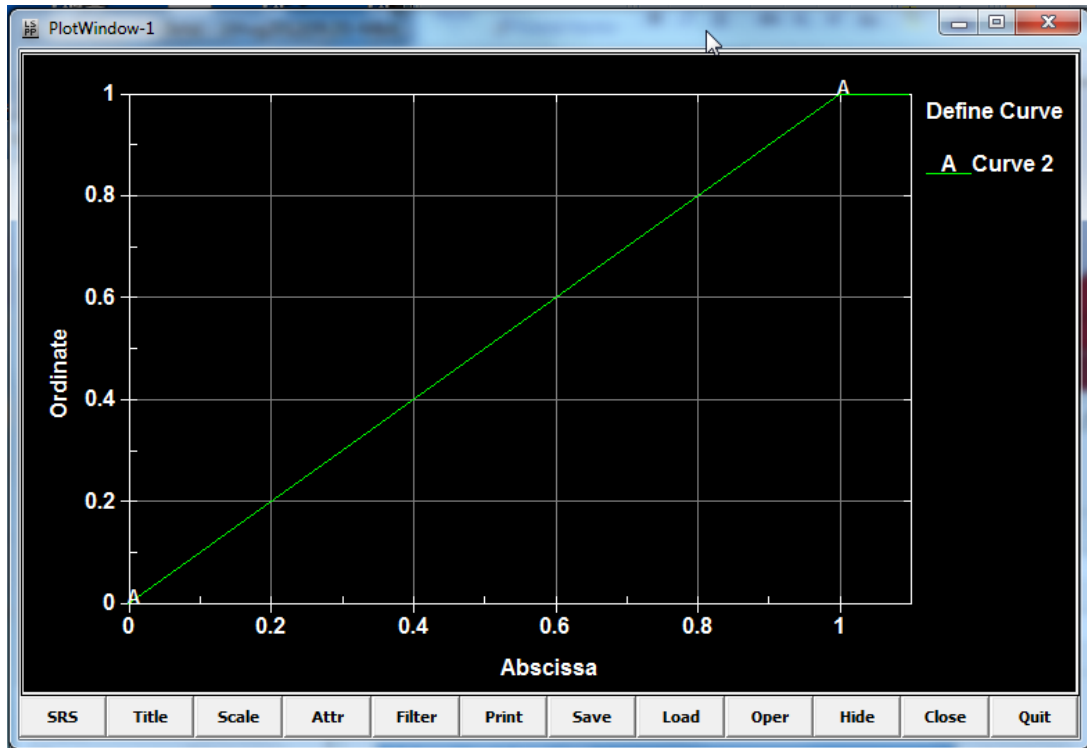
Now the table will look like:

A1	O1
1.100000	1.000000

1	0.0000e+000	0.0000e+000
2	1.0000e+000	1.0000e+000
3	1.1000e+000	1.0000e+000

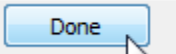
Hit **Accept**  and you will see **2 Ramp**  in the right hand side of the form.

To check the curve, hit the  button to see the curve plotted. You should see:



Now **Close** the plot window

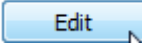
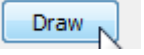
If you did not put the small plateau on the curve, it would drop to zero at the end and would give strange results. We will simulate 1 sec of time, so this curve has an X range of 0 to 1 (plus 10% we won't use). The vertical range is 1, but we will scale that to 18000N. If you planned to simulate 2 seconds, this curve would need an x range of 0 to 2.

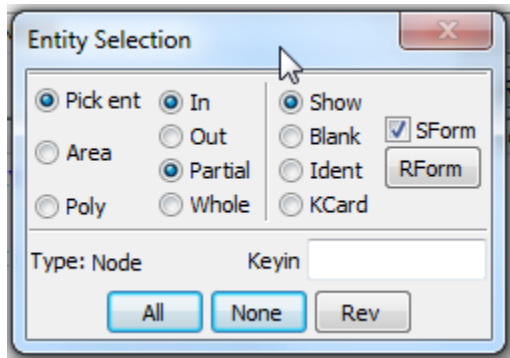
Now hit  to close the **Keyword Input Form**.


2) Find the Node number at the point for the load.

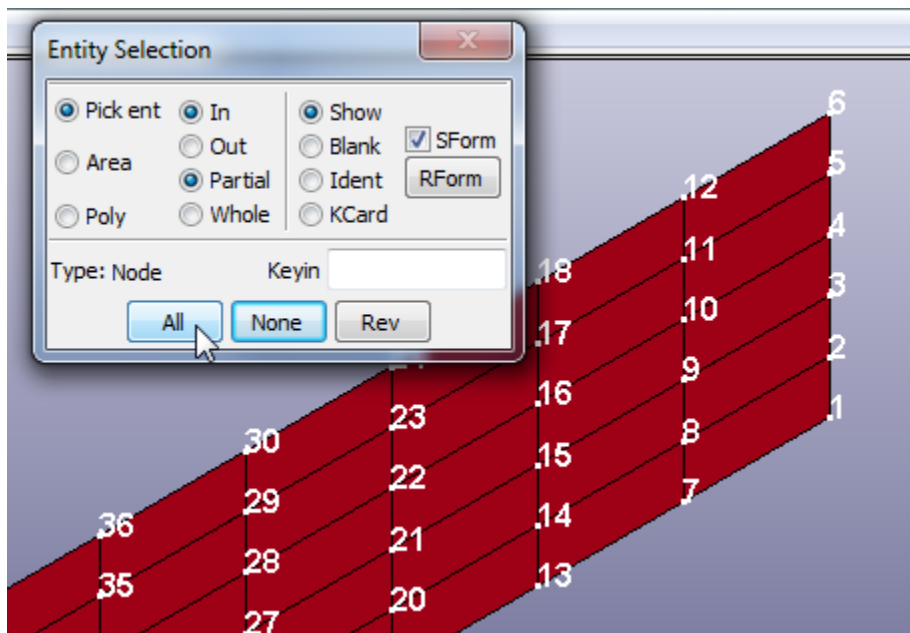
Go back to the **Keyword Manager** (select **Model** to simplify the task) and select **NODE** (and **NODE** again)



Now hit  and you will see the full list of nodes. You could edit the nodes, but we just want to see them. Select the  command on the **Keyword Input Form**. The form then shrinks and an **Entity Selection** window appears:



Click the  button to see all the nodes labeled:



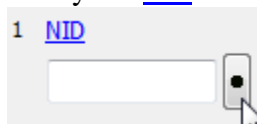
We will apply our force to the node with number 6.

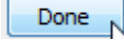
Hit  to remove the labels. Restore the **Keyword Input Form** window and hit .

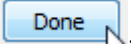
3) Define the force.

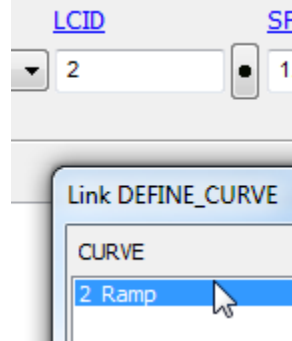
Return to the **Keyword Manager** window, select  and then .

Now select , and . Now you see the input form. Hit the black dot by the **NID** box:



and select node 6. and hit . In the **DOF** box select the **3** to specify the z direction.

Hit the black dot for the LCID box to specify the load curve. Select the ramp curve we defined earlier and hit .



Now scale the force by 18000. Type *-18000* into the SF box. This will mean that the load will ramp up from 0 to 18000N over the course of 1 second. And the load will act downwards (-z).

(Note - if you want to see help for one of the boxes - just put the cursor in the box and look at the bottom of the window)

hit  and .

Step 10: Specify the duration of the simulation.

Return to the Keyword Manager and select .

Scroll down to find  and then click .

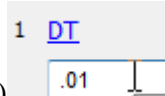
Put 1 in the ENDTIM box : 

hit  and .

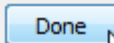
Step 11: Specify output frequency.

LSDyna will solve the problem with a very short timestep. There would likely be far too much output if you were to look at all the data for every solution timestep. Instead you can specify the frequency of the output. Return to the **Keyword Manager** and

select the **DATABASE** and **BINARY_D3PLOT** Keywords. and then click .

Specify an output timestep DT for the d3plot data of 0.01 seconds (10ms) 

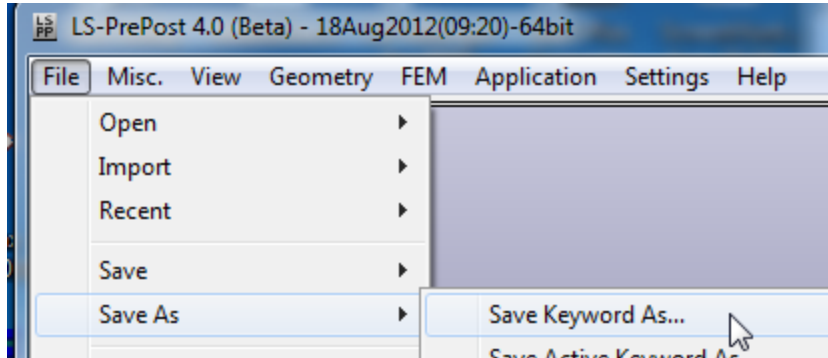
hit  and .

In the **Keyword Manager** window hit  to close it

Step 12: Save the .k file

Under the **File** menu select the **Save Keyword As** command and save the file as *Cant1.k* in its own folder (to keep the output files all in one place)

Note - you will have to type *Cant1.k* , as LSPrePost will not automatically add the .k extension.

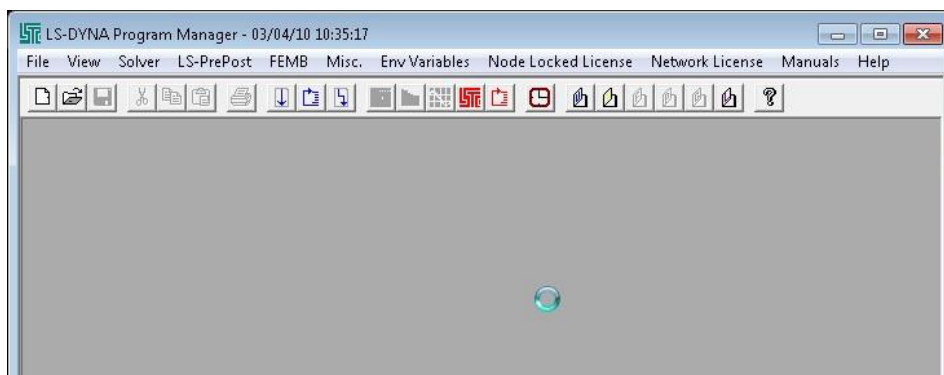


The .k file is a simple text file with all the problem information listed as either Keywords (lines starting with *) or associated data (no *) . There are also comments (lines beginning with \$#). The .k file that we have just created is in the appendix below. You should examine the .k file to see how the problem input data is represented.

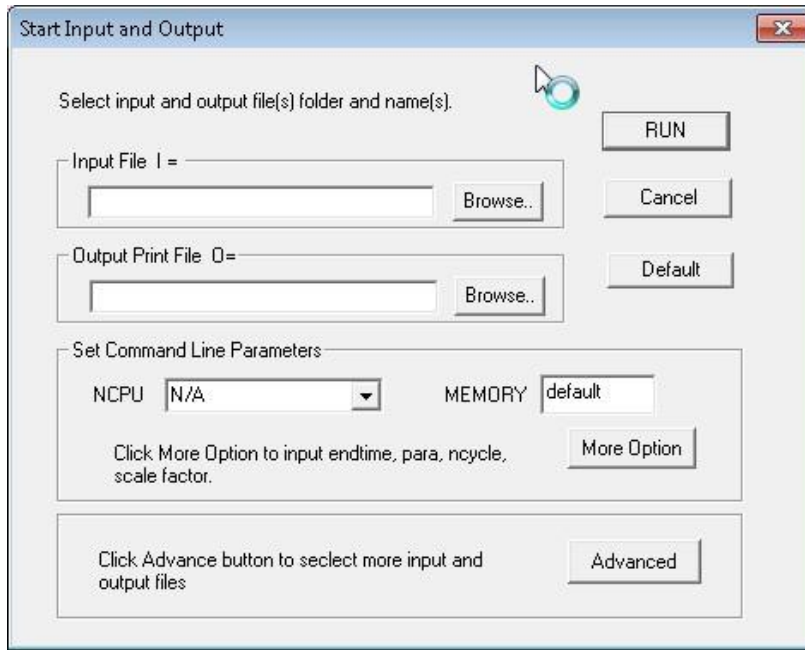
Step 13: Run the LS-Dyna Analysis



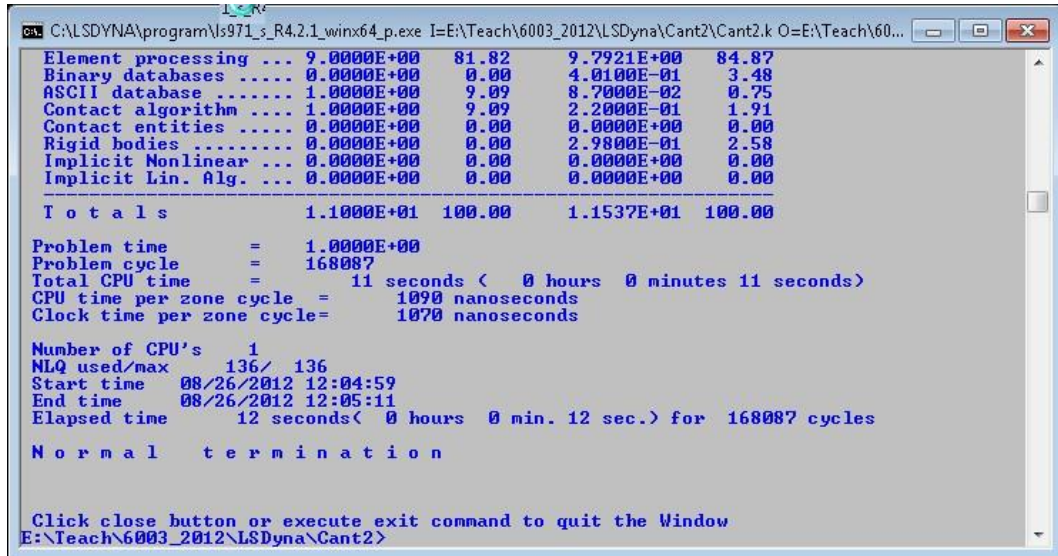
1) Click on the **LS-Dyna Manager** Program , which opens the window.:



In the top menu, select **Solver** and select **Start LS-Dyna** Analysis. Next you see the input screen. Use the **Browse** button to select the *Cant1.k* file. By default the same folder as the .k file is in will be the folder where the output is sent.

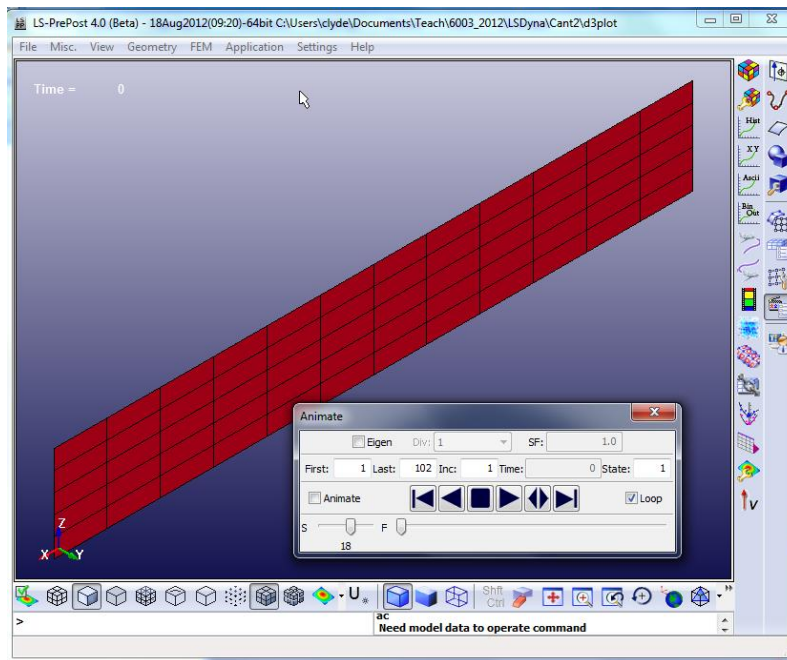


When the .k file is selected, press the **RUN** button. A command window will pop up showing the computation steps. When finished it should say **Normal termination** :



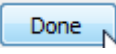


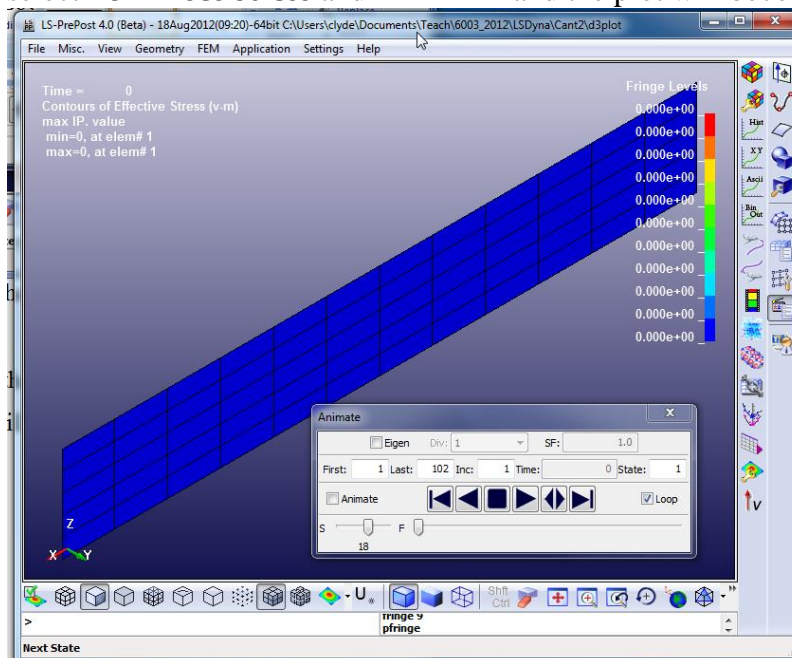
Step 14: Examine results of the LS-Dyna Analysis


- 1) Re-Open LS-PrePost.
- 2) From the **File** Menu, Select **Open** and **LS-Dyna Binary Plot**.
- 3) In the same folder as the .k file should be a file called **d3plot** - open it. Now you should see the screen as shown below.




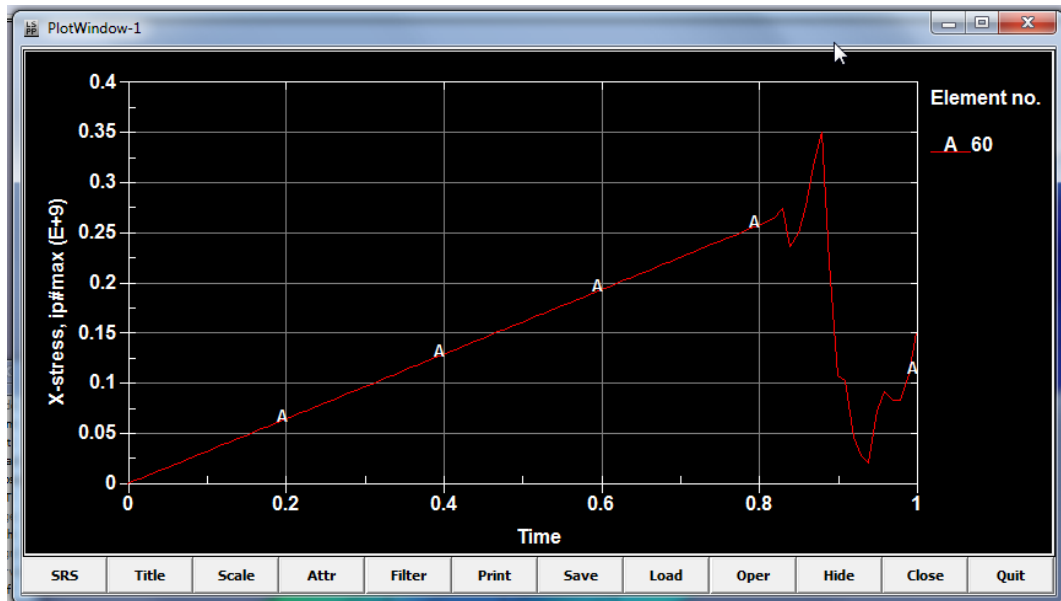
You can hit the Play button to animate the bar over the 1 second of analysis.

To see more data, hit the **Post** button , then hit the **Fringe Component**  select **Von Mises Stress** and **Done**  and the plot will become:



Now you will see the animated stresses. You can select **Static** under the **Fringe Range** button  to keep a constant range of stress during the animation.

4) There are many more results than can be plotted. For example click on the **History** button , select the **Element** option, select one element on the model, select one output (say **X-stress**) and hit **Plot**. You should see something like:



Summary of Steps


Step 1: describe and sketch the problem:

Step 2: estimate expected results (analytically):

Step 3: open LS-PrePost

Step 4: Directly create the FE model (nodes and element mesh)

element and mesh button: 

shape mesher button: 

Entity box: 4N_Shell

Step 5: Define the material properties

MAT to open the material options:

003-PLASTIC_KINEMATIC 

Step 6: Define the section properties

Section

Shell

Step 7: Apply the material and section properties to the model (to the 'Part')

Step 8: Apply boundary conditions (fix the end of the bar')

Boundary

Spc

Step 9: Apply a force to the free end of the bar.

Define a standard ramp curve:

Find the Node number at the point for the load.

LOAD

NODE_POINT

Step 10: Specify the duration of the simulation.

CONTROL

TERMINATION

Step 11: Specify output frequency.

DATABASE

BINARY_D3PLOT

Step 12: Save the .k file

Save Keyword As

Step 13: Run the LS-Dyna Analysis

should say **Normal termination** :

Step 14: Examine results of the LS-Dyna Analysis

Open LS-PrePost.

Open LS-Dyna Binary Plot.

d3plot

Appendix A - The .k file

```

$# LS-DYNA Keyword file created by LS-PrePost 4.0 (Beta) - 18Aug2012 (09:20)
$# Created on Aug-26-2012 (09:05:32)
*KEYWORD
*BOUNDARY_SPC_SET
$#   nsid      cid      dofx      dofy      dofz      dofrx      dofry      dofrz
    1          0          1          1          1          1          1          1
*SET NODE LIST TITLE
NODESET(SPC) 1
$#   sid      dal      da2      da3      da4      solver
    1      0.000      0.000      0.000      0.000MECH
$#   nid1      nid2      nid3      nid4      nid5      nid6      nid7      nid8
    73         74         75         76         77         78         0         0
*NODE
$#   nid      x      y      z      tc      rc
    1      0.000      0.000      0.000      0      0
    2      0.000      0.000      0.036000      0      0
    3      0.000      0.000      0.072000      0      0
    4      0.000      0.000      0.108000      0      0
    5      0.000      0.000      0.144000      0      0
    6      0.000      0.000      0.180000      0      0
    7      0.100000      0.000      0.000      0      0
    8      0.100000      0.000      0.036000      0      0
    9      0.100000      0.000      0.072000      0      0
    10     0.100000      0.000      0.108000      0      0
    11     0.100000      0.000      0.144000      0      0
    12     0.100000      0.000      0.180000      0      0
    13     0.200000      0.000      0.000      0      0
    14     0.200000      0.000      0.036000      0      0
    15     0.200000      0.000      0.072000      0      0
    16     0.200000      0.000      0.108000      0      0
    17     0.200000      0.000      0.144000      0      0
    18     0.200000      0.000      0.180000      0      0
    19     0.300000      0.000      0.000      0      0
    20     0.300000      0.000      0.036000      0      0
    21     0.300000      0.000      0.072000      0      0
    22     0.300000      0.000      0.108000      0      0
    23     0.300000      0.000      0.144000      0      0
    24     0.300000      0.000      0.180000      0      0
    25     0.400000      0.000      0.000      0      0
    26     0.400000      0.000      0.036000      0      0
    27     0.400000      0.000      0.072000      0      0
    28     0.400000      0.000      0.108000      0      0
    29     0.400000      0.000      0.144000      0      0
    30     0.400000      0.000      0.180000      0      0
    31     0.500000      0.000      0.000      0      0
    32     0.500000      0.000      0.036000      0      0
    33     0.500000      0.000      0.072000      0      0
    34     0.500000      0.000      0.108000      0      0
    35     0.500000      0.000      0.144000      0      0
    36     0.500000      0.000      0.180000      0      0
    37     0.600000      0.000      0.000      0      0
    38     0.600000      0.000      0.036000      0      0
    39     0.600000      0.000      0.072000      0      0
    40     0.600000      0.000      0.108000      0      0
    41     0.600000      0.000      0.144000      0      0
    42     0.600000      0.000      0.180000      0      0
    43     0.700000      0.000      0.000      0      0
    44     0.700000      0.000      0.036000      0      0
    45     0.700000      0.000      0.072000      0      0
    46     0.700000      0.000      0.108000      0      0
    47     0.700000      0.000      0.144000      0      0
    48     0.700000      0.000      0.180000      0      0
    49     0.800000      0.000      0.000      0      0
    50     0.800000      0.000      0.036000      0      0
    51     0.800000      0.000      0.072000      0      0
    52     0.800000      0.000      0.108000      0      0
    53     0.800000      0.000      0.144000      0      0
    54     0.800000      0.000      0.180000      0      0
    55     0.900000      0.000      0.000      0      0
    56     0.900000      0.000      0.036000      0      0
    57     0.900000      0.000      0.072000      0      0
    58     0.900000      0.000      0.108000      0      0
    59     0.900000      0.000      0.144000      0      0
    60     0.900000      0.000      0.180000      0      0
    61     1.000000      0.000      0.000      0      0
    62     1.000000      0.000      0.036000      0      0
    63     1.000000      0.000      0.072000      0      0
    64     1.000000      0.000      0.108000      0      0
    65     1.000000      0.000      0.144000      0      0
    66     1.000000      0.000      0.180000      0      0
    67     1.100000      0.000      0.000      0      0
    68     1.100000      0.000      0.036000      0      0
    69     1.100000      0.000      0.072000      0      0
    70     1.100000      0.000      0.108000      0      0
    71     1.100000      0.000      0.144000      0      0
    72     1.100000      0.000      0.180000      0      0
    73     1.200000      0.000      0.000      0      0
    74     1.200000      0.000      0.036000      0      0
    75     1.200000      0.000      0.072000      0      0
    76     1.200000      0.000      0.108000      0      0
    77     1.200000      0.000      0.144000      0      0
    78     1.200000      0.000      0.180000      0      0
*DATABASE_BINARY_D3PLOT
$#   dE      lcdt      beam      npltc      psetid
    0.010000      0          0          0          0
$#   iocpt
    0
*SECTION_SHELL_TITLE
Shell Section
$#   secid      elform      shrff      nip      propt      qr/irid      icomp      setyp
    2          2      0.833300      5          1          0          0          1
$#   t1      t2      t3      t4      nloc      marea      idof      edgset
    0.010000      0.010000      0.010000      0.010000      0.000      0.000      0.000      0
*CONTROL_TERMINATION
$#   endTim      endcyc      dtmin      endeng      endmas
    1.000000      0          0.000      0.000      0.000
*ELEMENT_SHELL
$#   eid      pid      n1      n2      n3      n4      n5      n6      n7      n8
    1          1          7          8          2          1          0          0          0          0
    2          1          8          9          3          2          0          0          0          0
    3          1          9          10         4          3          0          0          0          0
    4          1          10         11         5          4          0          0          0          0
    5          1          11         12         6          5          0          0          0          0
    6          1          13         14         8          7          0          0          0          0
    7          1          14         15         9          8          0          0          0          0
    8          1          15         16         10         9          0          0          0          0
    9          1          16         17         11         10         0          0          0          0
    10         1          17         18         12         11         0          0          0          0
    11         1          18         19         13         12         0          0          0          0
    12         1          19         20         14         13         0          0          0          0
    13         1          20         21         15         14         0          0          0          0
    14         1          21         22         16         15         0          0          0          0
    15         1          22         23         17         16         0          0          0          0
    16         1          23         24         18         17         0          0          0          0
    17         1          24         25         19         18         0          0          0          0
    18         1          25         26         20         19         0          0          0          0
    19         1          26         27         21         20         0          0          0          0
    20         1          27         28         22         21         0          0          0          0
    21         1          28         29         23         22         0          0          0          0
    22         1          29         30         24         23         0          0          0          0
    23         1          30         31         25         24         0          0          0          0
    24         1          31         32         26         25         0          0          0          0
    25         1          32         33         27         26         0          0          0          0
    26         1          33         34         28         27         0          0          0          0
    27         1          34         35         29         28         0          0          0          0
    28         1          35         36         30         29         0          0          0          0
    29         1          36         37         31         30         0          0          0          0
    30         1          37         38         32         31         0          0          0          0
    31         1          38         39         33         32         0          0          0          0
    32         1          39         40         34         33         0          0          0          0
    33         1          40         41         35         34         0          0          0          0
    34         1          41         42         36         35         0          0          0          0
    35         1          42         43         37         36         0          0          0          0
    36         1          43         44         38         37         0          0          0          0
    37         1          44         45         39         38         0          0          0          0
    38         1          45         46         40         39         0          0          0          0
    39         1          46         47         41         40         0          0          0          0
    40         1          47         48         42         41         0          0          0          0
    41         1          48         49         43         42         0          0          0          0
    42         1          49         50         44         43         0          0          0          0
    43         1          50         51         45         44         0          0          0          0
    44         1          51         52         46         45         0          0          0          0
    45         1          52         53         47         46         0          0          0          0
    46         1          53         54         48         47         0          0          0          0
    47         1          54         55         49         48         0          0          0          0
    48         1          55         56         50         49         0          0          0          0
    49         1          56         57         51         50         0          0          0          0
    50         1          57         58         52         51         0          0          0          0
    51         1          58         59         53         52         0          0          0          0
    52         1          59         60         54         53         0          0          0          0
    53         1          60         61         55         54         0          0          0          0
    54         1          61         62         56         55         0          0          0          0
    55         1          62         63         57         56         0          0          0          0
    56         1          63         64         58         57         0          0          0          0
    57         1          64         65         59         58         0          0          0          0
    58         1          65         66         60         59         0          0          0          0
    59         1          66         67         61         60         0          0          0          0
    60         1          67         68         62         61         0          0          0          0
    61         1          68         69         63         62         0          0          0          0
    62         1          69         70         64         63         0          0          0          0
    63         1          70         71         65         64         0          0          0          0
    64         1          71         72         66         65         0          0          0          0
    65         1          72         73         67         66         0          0          0          0
    66         1          73         74         68         67         0          0          0          0
    67         1          74         75         69         68         0          0          0          0
    68         1          75         76         70         69         0          0          0          0
    69         1          76         77         71         70         0          0          0          0
    70         1          77         78         72         71         0          0          0          0
*DEFINE_CURVE_TITLE
Ramp
$#   lcid      sidr      sfa      sfo      offa      offo      dattyp
    2          0      1.000000      1.000000      0.000      0.000      0
$#   al      ol
    0.000      0.000
    1.000000      1.000000
    1.100000      1.000000
*MAT_PLASTIC_KINEMATIC_TITLE
Steel material
$#   mid      ro      e      pr      sigy      etan      beta
    1      7850.00002.0700E+11      0.3000000      3.0000E+8      1.0000E+9      0.000
$#   src      srp      fs      vp
    0.000      0.000      0.000      0.000
*PART
$# title
Cantilever
$#   pid      secid      mid      eosid      hgid      grav      adpopt      tmid
    1          2          1          0          0          0          0          0
*LOAD_NODE_POINT
$#   nid      dof      lcid      sf      cid      m1      m2      m3
    6          3          2-18000.000      0          0          0          0          0
*END

```



Self Study Exercises:

Student: _____

For each of these exercises, be prepared to show the instructor your results.

Exercise #1 – Change the material model . Open the .k file. Find and delete the material model. Now create a new MAT model using 001-Elastic (ie no yielding) re-save the model as Cant2 in a new folder (make sure output goes to new folder). Run the model and fill in table below for time = 1 sec.

Deflection at end	Original model	MAT 001-Elastic
deflection at end [mm]:		
Eqv. Stress at base [MPa]		
Eqv.Stress at end [MPa]		
Comments: ?		

Ex#1 Initials of Instructor _____

Exercise #2 – Redo the analysis starting with a CAD step. Start by drawing a plane and then use the **Auto mesher** to make a mesh of triangles. Repeat all the rest of the steps.

Deflection at end	With Triangular mesh
deflection at end [mm]:	
Eqv. Stress at base [MPa]	
Eqv.Stress at end [MPa]	
Comment:	

Ex#2 Initials of Instructor _____